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Construction Costs of Six Landfill Cover Designs

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Abstract

A large-scale field demonstration comparing and contrasting final landfill cover designs has been constructed and is currently being monitored. Four alternative cover designs and two conventional designs (a RCRA Subtitle 'D' Soil Cover and a RCRA Subtitle 'C' Compacted Clay Cover) were constructed side-by-side for direct comparison. The demonstration is intended to evaluate the various cover designs based on their respective water balance performance, ease and reliability of construction, and cost. This paper provides an overview of the construction costs of each cover design.

Engineering Background

The US Department of Energy (DOE) is in the midst of a major clean-up effort of their facilities that is expected to cost billions of dollars. These cost estimates; however, are based on “state-of-the-art” technologies, of which many are inadequate. Consequently, work has begun on the development or improvement of environmental restoration and management technologies. One particular area being researched is landfill covers. As part of their ongoing environmental restoration activities, the DOE has many radioactive, hazardous, mixed waste, and sanitary landfills to be closed in the near future. These sites, as well as mine and mill tailings piles and surface impoundments, all require either remediation to a ‘clean site’ status or capping with an engineered cover upon closure (Hakonson et al., 1994). Additionally, engineered covers are being considered as an interim measure to be placed on contaminated sites until they can be remediated. The Alternative Landfill Cover Demonstration (ALCD) is a large-scale field test at Sandia National Laboratories, located on Kirtland Air Force Base in Albuquerque, New Mexico. Its intent is to compare and document the performance of alternative landfill cover technologies of various costs and complexities for interim stabilization and/or final closure of landfills in arid and semi-arid environments. The test covers are constructed side-by-side for direct comparison based on their performance, cost, and ease of construction. The ALCD is not intended to showcase any one particular cover system. The focus of this project is to provide the necessary tools; i.e., cost, construction and performance data, to the public and regulatory agencies so that design engineers will have less expensive, regulatory acceptable alternatives to the conventional cover designs.

The test covers are each 13 m wide by 100 m long. The 100 m dimension was chosen because it is representative of hazardous and mixed waste landfills found throughout the DOE complex (approximately 2 acres in surface area). All covers were constructed with a 5% slope in all layers. The slope lengths are 50 m each (100 m length crowned at the middle with half of the length - 50 m - sloping to the east and the other half toward the west). The western slope is monitored under ambient conditions (passive monitoring). A sprinkler system was installed in the eastern slope of each cover to facilitate stress testing of the covers (active monitoring).

Continuous water balance and meteorological data is currently being obtained. It will be actively collected for a minimum five-year post construction period. In addition, periodic measurements of vegetation cover, biomass, leaf area index, and species composition are being taken.

General Costs of Construction

The covers were independently designed. The designs were packaged into a set of construction bid documents that included drawings and specifications for each test cover. The covers were divided into two separate bid packages. Preparation of these bid packages including design was not included in the costs of the covers presented in this paper. Phase I was bid in FY95 while Phase II, due to funding constraints, was bid in FY96. The phase I covers built in FY95 include a conventional RCRA Subtitle ‘D’ Soil Cover, a conventional RCRA Subtitle ‘C’ Compacted Clay Cover, and the Geosynthetic Clay Liner (GCL) Cover. The phase II covers built in FY96 include the Capillary Barrier, Anisotropic Barrier, and Evapotranspiration (ET) Cover.

Each phase of construction was competitively bid with the low bidder receiving a firm fixed price contract. The costs to construct each cover design were broken out of the overall contract with the help of the general contractor and applicable subcontractors. The entire process including the bid package preparation, bid

process, contract award, and construction activities were performed similar to that on an actual landfill cover project.

Costs for each cover design were developed as follows: (1) common costs such as mobilizing, demobilizing, and subgrade preparation were evenly assessed to each cover; (2) all other costs such as materials and labor were carefully allocated to each cover design. A summary of the cost per surface square meter for each cover is presented in figure 7. A detailed break down of costs for each respective cover is presented in Attachment A.

Conventional EPA Landfill Covers

Two conventional cover designs were installed to provide a baseline for comparison of the alternative cover designs. Conventional Test Cover 1 (Figure 1) is a basic Soil Cover that is typically installed over sanitary municipal landfills. This cover design meets minimum requirements set forth for RCRA Subtitle 'D' governed landfills (US Dept. of Energy, 1993). This cover is 60 cm thick. It is constructed of essentially two layers. The bottom layer is a 45 cm thick compacted soil barrier layer. Only native soil was used in this layer. The soil was compacted 'wet of optimum' to 95% of maximum dry density. The soil was placed as specified to meet the maximum 1×10^{-5} cm/sec saturated hydraulic conductivity requirement of Subtitle 'D' regulated facilities. The top vegetation layer is 15 cm of topsoil loosely laid. This layer provides for vegetation growth and erosion protection.

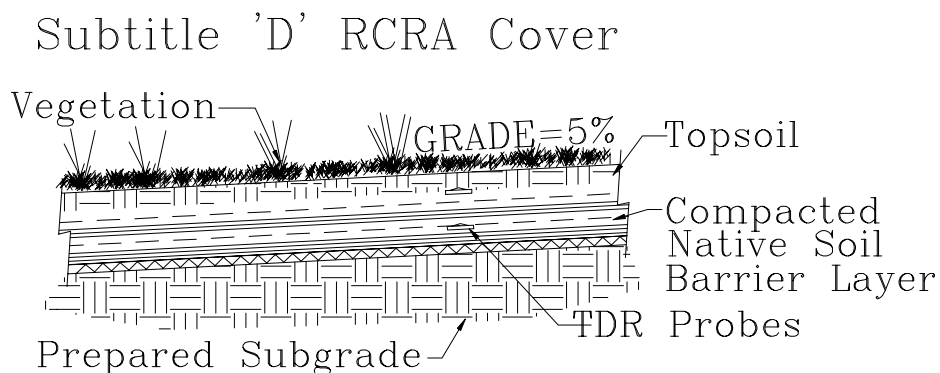


Figure 1. Conventional Test Cover 1

The Soil Cover was the simplest and least expensive of all of the covers installed. The majority of the costs include obtaining and placing the 60 cm thick fill soil. The bottom 45 cm barrier soil layer required compaction. This barrier layer was installed in three 15 cm compacted lift depths. Compaction was relatively easily achieved using a vibratory compactor. The cost of construction for this cover design was \$51.40 per square meter. The largest potential variability in cost for this design will be with obtaining the fill soil. If acceptable on-site soil can be used from a nearby borrow pit with little pretreatment, a substantial cost savings would be obtained compared to purchasing the fill and having it trucked on-site.

This cover, however, is generally considered inadequate at controlling infiltration into the underlying waste it is designed to isolate from the surrounding environment. An EPA sponsored study revealed that a large percentage of landfills utilizing this cover design have failed (US Environmental Protection Agency, 1988).

Conventional Test Cover 2 (Figure 2) is a Compacted Clay Cover that is generally installed over hazardous waste landfills. This cover was designed and installed to meet minimum requirements set forth for RCRA Subtitle 'C' regulated landfills (EPA, 1991). It is 1.5 m thick. The typical profile for this cover consists of three layers. The bottom layer is a 60 cm thick barrier layer. The barrier layer's primary purpose is to prevent the downward movement of water into underlying waste. Laboratory tests revealed that the native soil required amendment to meet the saturated hydraulic conductivity requirement (maximum of 1×10^{-7} cm/sec). It was constructed of native soil mixed with 6% bentonite by weight. The lifts were compacted to a minimum of 98% of maximum dry density (ASTM D698). During fill and compaction, the soil was kept at a water content 'wet of optimum' so as to remold the soil. The combination of the compaction requirements, soil amendment, and placement ('wet of optimum') was necessary to yield a maximum hydraulic conductivity of 1×10^{-7} cm/sec. Constructing the barrier layer was very difficult. The most economical means of obtaining the bentonite was having it trucked in from Wyoming. It arrived in two-ton bags. The bentonite was then evenly mixed with the soil prior to its installation in the cover. The purchase and mixing of this bentonite into the soil increased the cost of the cover by 14% versus using unamended soil. After mixing, the amended soil was wetted, placed on the cover and compacted. The moist amended soil proved to be much more difficult to work with than the plain native soil. It was sticky to spread and slippery to drive on. The compaction was extremely difficult to achieve. It was compacted in four approximately 15 cm compacted depth lifts. Each lift in the barrier layer took about three times more effort to achieve compaction requirements than did the lifts in Conventional Test Cover 1. Substantial quality control was required to ensure the material and placement was as specified.

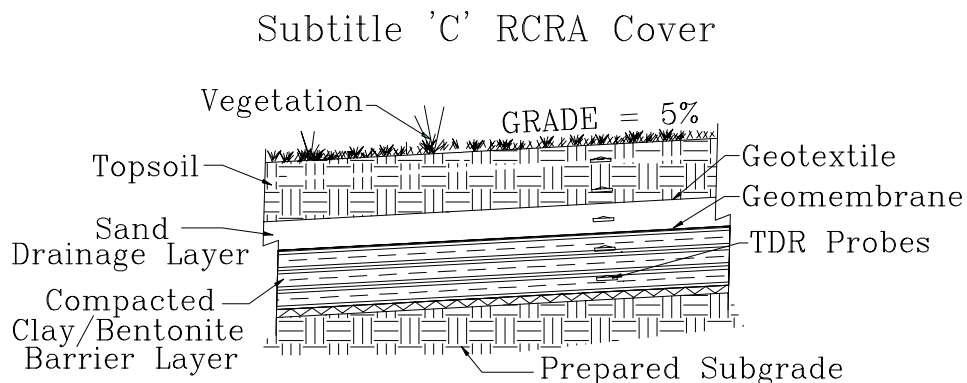


Figure 2. Conventional Test Cover 2

A 40 mil linear low density polyethylene geomembrane was placed directly on top of the clay barrier layer. The surface of the clay barrier layer was smooth-roll compacted and prepared to allow for intimate contact between it and the under-surface of the geomembrane to essentially obtain a composite barrier layer. The geomembrane was purchased and trucked to the site. The cost of the geomembrane includes not only the material cost but shipping costs. It can get very expensive for a small project if a partial load is shipped rather than a full load because the full cost of that truck's time and mileage will be billed to that shipment. This is true for all geosynthetics used. On this project, we were able to have full loads shipped. The installation of the geomembrane required spreading it in place and heat welding the seams. All seams were tested for adequacy. Any and all defects found in the geomembrane were patched and tested for adequacy. The total installed cost for the geomembrane was about \$10.12 per square meter (\$0.94 per square foot).

The middle layer is a 30 cm thick drainage layer. The primary purpose of the drainage layer is to quickly route any water that has passed through the vegetation layer laterally to collection drains normally located at

the perimeter of the landfill. This layer was constructed of sand placed directly on the geomembrane. The average hydraulic conductivity of the sand installed was 1×10^{-1} cm/sec which is an order of magnitude better than the minimum 1×10^{-2} cm/sec called for in Subtitle 'C'. It was trucked to the site and spread in place in a single lift. The sand used was a common material called washed concrete sand. Its cost from the supplier was about \$7.70 per ton.

A nonwoven polyester needlepunched geotextile was placed directly on top of the sand drainage layer. This served as a filter between the drainage layer material and top layer. The rolls of geotextile were placed on the sand and rolled into place. Joints were simply overlapped; no physical seaming was performed. This geotextile was installed at a cost of about \$3 per square meter (\$0.28 per square foot).

The top layer is a 60 cm thick vegetation layer comprised of loosely laid soil. This layer's primary purpose is to provide for vegetation growth, erosion protection, and protect the underlying layers from such events as harmful freeze/thaw cycles. It allows for storage of infiltrated water that can later be evaporated. It is 45 cm of native soil covered by 15 cm of topsoil.

The Compacted Clay Cover was by far the most difficult and expensive test cover to install. Its construction cost amounted to \$157.54 per square meter.

Alternative Designs

Any and all compaction of soil in the alternative designs was compacted 'dry of optimum' rather than 'wet of optimum' as currently recommended with the conventional covers (EPA, 1993). This was done in an effort to mitigate the potential for desiccation cracking.

Alternative Test Cover 1 (Figure 3) is a Geosynthetic Clay Cover identical to the conventional compacted clay cover with the exception that the problematic clay barrier layer was replaced with a manufactured sheet, a geosynthetic clay liner (GCL), installed in its place.

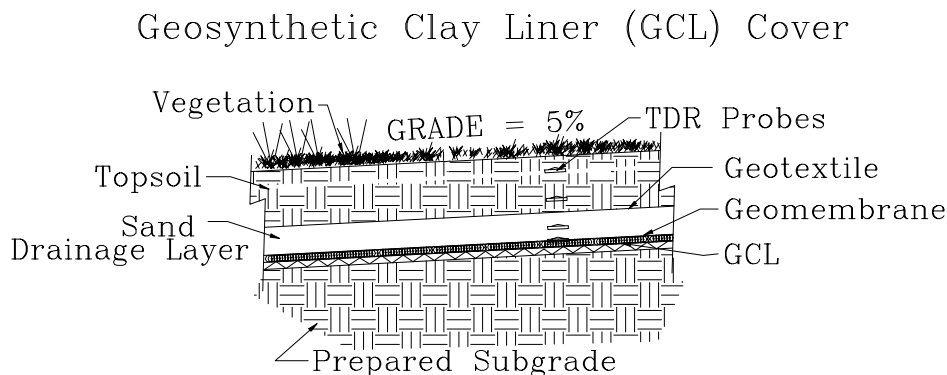


Figure 3. Alternative Test Cover 1

The GCL is the bottom barrier layer covered with a geomembrane, drainage layer and vegetation layer, respectively. The GCL sheet installed is a composite of two nonwoven fabrics sandwiching a layer of bentonite. The hydraulic conductivity of the GCL is 5×10^{-9} cm/sec.

Replacing the 60 cm thick clay (amended soil) barrier layer with a GCL drastically reduced the cost and difficulty of construction. The GCL was purchased and trucked to the site. The rolls were simply rolled into place. Joints were overlapped with no physical seaming. Most of the cost difference between this cover and that of the Conventional Test Cover 2 is the savings gained by installing a GCL rather than a 2 foot thick compacted clay layer. Other related savings comes from reduced construction time and thus reduced job overhead expenses as well as less job foremen and survey crew time. The topsoil, sand drainage layer, geotextile, and geomembrane were installed similarly to that in the Compacted Clay Cover. The construction cost of this cover design was \$89.99 per square meter.

Alternative Test Cover 2 (Figure 4) is a Capillary Barrier. The Capillary Barrier comprised of a fine-grained layer of soil placed over a coarse-grained layer emphasizes a sufficient contrast between the hydraulic conductivity's of the fine-grained layer versus the coarse-grained layer. This contrast lends to the effect that flow through the cover is greatly slowed under unsaturated conditions.

This cover system consists of 4 primary layers: (1) a surface or topsoil layer; (2) an upper drainage layer; (3) a barrier soil layer; and (4) a lower drainage layer. The topsoil layer is 30 cm thick. This surface layer is placed to enhance evapotranspiration, protect against desiccation of the barrier soil layer, and provide a medium for growth of vegetation. This vegetation increases evapotranspiration and protects against surface erosion. The upper lateral drainage layer is 22 cm of gravel overlain by 8 cm of sand. The sand serves as a graded filter to prevent topsoil from clogging the drainage layer. The gravel allows for lateral drainage of any water that has percolated through the topsoil. The barrier soil layer and lower drainage layer comprise the capillary barrier. The barrier soil layer is compacted soil 45 cm thick. The lower drainage layer is 30 cm of sand.

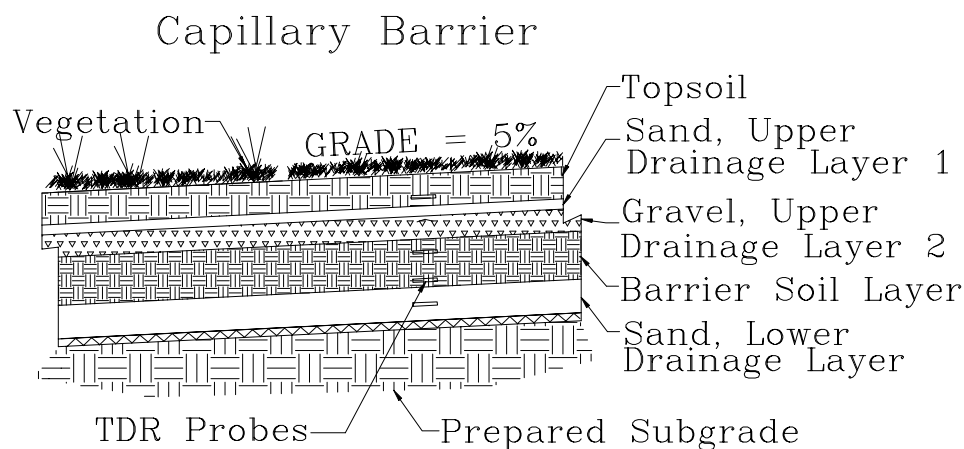


Figure 4. Alternative Test Cover 2

The Capillary Barrier was installed without any geosynthetic materials. The lower sand drainage layer is washed concrete sand. It was placed in one lift. The barrier soil layer was then placed on the sand. The first lift of the soil had to be carefully placed so as to keep a smooth uniform transition between the soil and sand. In order to minimize weight and thus not disturb the interface between the soil and sand, a small bulldozer with steel tracks was used to spread the soil. This took longer than using larger equipment with rubber tires and was slightly more expensive. The soil was placed in three 15 cm deep lifts. Compaction was relatively easily achieved. It was compacted to a minimum of 95% of maximum dry density. The gravel upper drainage layer was then placed in one lift. It was clean pea gravel, a readily available material. A sand upper drainage layer was then placed in one lift on the gravel. Again, special care was required to keep a smooth interface between the sand and gravel. The small bulldozer

was again used to spread the sand into place. The 8 cm thick layer of sand called for in the design proved impractical. Fifteen cm of sand seemed to be the thinnest practical depth. The construction cost of this cover design was \$92.64 per square meter.

Alternative Test Cover 3 (Figure 5) is referred to as the Anisotropic Barrier. The design of the Anisotropic Barrier attempts to limit downward movement of water while encouraging lateral movement of water. This cover is composed of a layering of capillary barriers. The various layers are enhanced by varying soil properties and compaction techniques that lead to the anisotropic properties of the cover.

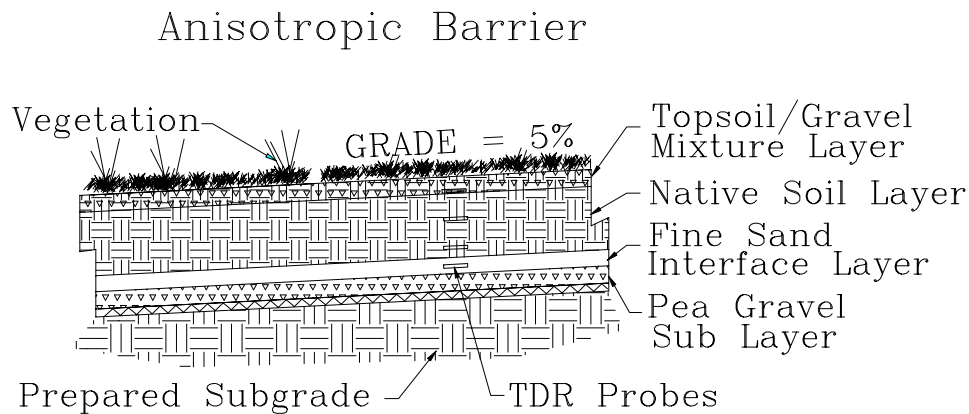


Figure 5. Alternative Test Cover 3

This cover system consists of 4 layers: (1) a top vegetation layer; (2) a cover soil layer; (3) an interface layer; and (4) a sublayer. The vegetation layer is 15 cm thick. It is comprised of a mixture of local topsoil and pea gravel. The gravel to soil mixture by weight was 25%. This layer encourages evapotranspiration, allows for vegetation growth, and reduces surface erosion. The cover soil layer is 60 cm of native soil. Its function is to allow for water storage and eventual evapotranspiration, as well as, serve as a rooting medium. The interface layer is 15 cm of fine sand. This layer serves as a filter between the overlying soil and the underlying gravel. It also serves as a drainage layer to laterally divert water that has percolated through the cover soil. The sublayer is 15 cm of pea gravel. It serves as a capillary break. The interface layer and sublayer combined also serve a dual purpose as bio-barriers.

The Anisotropic Barrier was installed without any geosynthetic materials. The pea gravel sublayer was installed in one lift. The same clean pea gravel as that used on the Capillary Barrier was used on this cover. The sand interface layer was installed in one lift by the small steel tracked bulldozer to maintain the clean interface between the sand and gravel. The sand was also washed concrete sand. The native soil layer was then installed in one lift on the sand. It was also spread with the small bulldozer. The only compaction on this soil layer was due to equipment traffic on it during construction. The topsoil/gravel layer was mixed adjacent to the cover then trucked on and spread in one lift. The gravel used was clean pea gravel. The construction cost of this cover design was \$75.26 per square meter.

Alternative Test Cover 4 (Figure 6) is referred to as an Evapotranspiration (ET) Cover. The ET Cover is a soil cover with an engineered vegetative covering. This cover encourages water storage and enhances ET. It is 90 cm thick. The bottom 75 cm was compacted while the top 15 cm of topsoil was loosely placed. The soil allows for water storage, which when combined with the vegetation will increase, evapotranspiration. A thin layer of gravel was then spread on the surface. This layer is about an inch thick and serves to reduce surface erosion and provide for better vegetation establishment.

Evapotranspiration Soil Cover

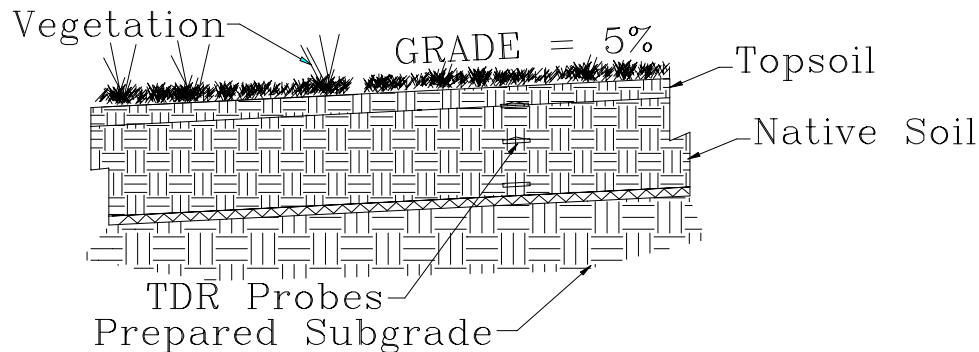


Figure 6. Alternative Cover 4

The ET Cover was constructed similarly to the RCRA Subtitle 'D' Soil Cover. The RCRA Subtitle 'D' Soil Cover's shallow depth is one of the primary reasons for its inadequacy. Computer modeling revealed that if the depth were increased to 90 cm in the Albuquerque climate, this would essentially eliminate water infiltration into underlying waste. The ET Cover was easily constructed placing five 15 cm thick compacted depth lifts overlain by one lift of topsoil. The construction cost of this cover was \$73.89 per square meter.

Alternative Acceptance

A study (Wentz, 1989) performed by the University of North Dakota concluded that the two most important factors affecting which hazardous waste management technology are to be used are that they are regulatorily acceptable and they are less expensive than past choices.. Great effort has gone into obtaining regulatory involvement and eventual acceptance of the technologies involved in this project. Because each of the alternative designs is less expensive and easier to construct than the conventional Compacted Clay Cover and much more effective than the plain Soil Cover they should be widely accepted.

Expected Benefits

The probable outcome of this demonstration is the acceptance of alternative cover designs that are significantly less costly than conventional designs. Given the thousands of acres of buried waste sites to be covered, the payoff from this demonstration may be on the order of many millions of dollars in savings.

Acknowledgments

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Summary

The construction costs for each of the six landfill cover designs is shown below in figure 7. These costs may vary depending on the size and location of the site.

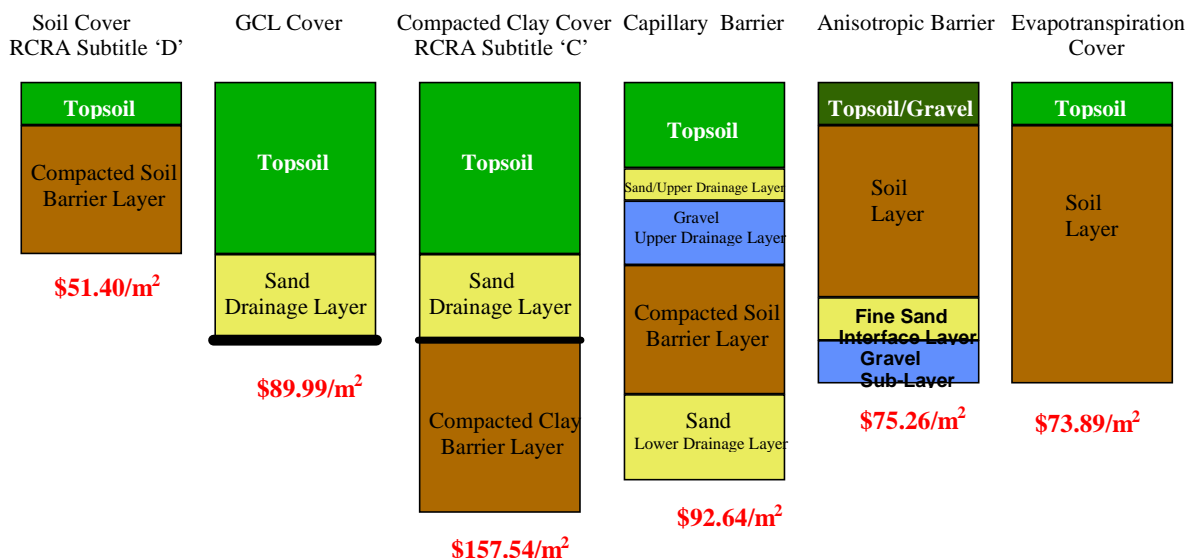


Figure 7. Summary of Construction Costs

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- 1 MS0719 D.A. Padilla, 6131
- 1 MS0724 J. B. Woodard, 6000
- 1 MS1132 W. B. Cox, 6100
- 1 MS1132 R. E. Fate, 6132
- 1 MS1147 F. B. Nimick, 6133
- 1 MS1148 G. L. Peace,
- 2 MS0619 Review & Approval Desk, 12690 for DOE/OSTI
- 2 MS0899 Technical Library, 4916
- 1 MS9018 Central Technical Files, 8940-2

Attachment A

	A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T
1	Basic Construction Costs for Cover Portion Only																			
2																				
3																				
4																				
5	Work Item Description	Units	Labor	Material	Equipment	Labor	Material	Equipment	Quantity	Totals	Quantity	Totals	Quantity	Totals	Quantity	Totals	Quantity	Totals	Quantity	Totals
6	Engineered Fill Soil-Banier	ton		\$7.48		\$0.00	\$10.36	\$0.00	1395	\$14,358.86		\$0.00	1548	\$19,145.28	1395	\$14,358.86	1548	\$19,145.28	2310	\$23,931.60
7	compaction	day	\$200.00		\$991.56	\$340.00	\$0.00	\$1,857.75	3	\$4,193.25		\$0.00	12	\$16,772.99	3	\$4,193.25		\$0.00	5	\$8,986.75
8	placement	day	\$200.00		\$955.10	\$340.00	\$0.00	\$1,858.61	3	\$4,171.83		\$0.00	4	\$5,582.44	3	\$4,171.83	4	\$5,582.44	5	\$8,953.05
9	grade	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23	3	\$3,792.69		\$0.00	4	\$5,958.92	3	\$3,792.69	4	\$5,958.92	5	\$8,321.16
10	Engineered Fill Soil-Topsoil	ton		\$7.48		\$0.00	\$10.36	\$0.00	452	\$4,786.32	1040	\$19,145.28	1040	\$19,145.28	924	\$8,572.64	347	\$3,594.92	452	\$4,786.32
11	placement	day	\$200.00		\$955.10	\$340.00	\$0.00	\$1,858.61	1	\$1,390.81	3	\$4,171.83	3	\$4,171.83	2	\$2,791.22	1	\$1,390.81	1	\$1,390.81
12	grade	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23	1	\$1,264.23	3	\$3,792.69	3	\$3,792.69	2	\$2,528.46	1	\$1,264.23	1	\$1,264.23
13	Sand	ton		\$7.78		\$0.00	\$10.78	\$0.00	864	\$9,213.92	864	\$9,213.92	1296	\$12,970.88	432	\$4,656.96				
14	placement	day	\$200.00		\$955.10	\$340.00	\$0.00	\$1,858.61		\$0.00	1	\$1,390.81	1	\$1,390.81	2	\$2,791.22	1	\$1,390.81		\$0.00
15	grade	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23		\$0.00	1	\$1,264.23	1	\$1,264.23	2	\$2,528.46	1	\$1,264.23		\$0.00
16	Pea Gravel	ton		\$10.85		\$0.00	\$15.19	\$0.00		\$0.00		\$0.00		\$0.00	432	\$6,562.80	540	\$8,202.80		\$0.00
17	placement	day	\$200.00		\$955.10	\$340.00	\$0.00	\$1,858.61		\$0.00		\$0.00		\$0.00	1	\$1,390.81	1	\$1,390.81		\$0.00
18	grade	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23		\$0.00		\$0.00		\$0.00	1	\$1,264.23	1	\$1,264.23		\$0.00
19	mixing	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23		\$0.00		\$0.00		\$0.00		\$0.00	1	\$1,264.23		\$0.00
20	Gravel Mulch	ton		\$9.88		\$0.00	\$13.44	\$0.00		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00	53	\$712.32
21	placement	day	\$200.00			\$340.00	\$0.00	\$0.00		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00	0.5	\$176.00
22	grade	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00	0.5	\$832.12
23	Subgrade Preparation					\$0.00	\$0.00	\$0.00		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00
24	remove rock & debris	day	\$120.00		\$20.00	\$204.00	\$0.00	\$22.00	1	\$226.00	1	\$226.00	1	\$226.00	1	\$226.00	1	\$226.00	1	\$226.00
25	grade	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23	1	\$1,264.23	1	\$1,264.23	1	\$1,264.23	1	\$1,264.23	1	\$1,264.23	1	\$1,264.23
26	compact	day	\$200.00		\$991.56	\$340.00	\$0.00	\$1,857.75	1	\$1,397.75	1	\$1,397.75	1	\$1,397.75	1	\$1,397.75	1	\$1,397.75	1	\$1,397.75
27	Geomembrane, 40ml LLDPE	sf	\$0.50	\$0.00	\$0.00	\$0.95	\$0.00	\$0.00		\$0.00	13200	\$11,220.00	13200	\$11,220.00		\$0.00		\$0.00		\$0.00
28	GCL	sf	\$0.75	\$0.00	\$0.00	\$1.28	\$0.00	\$0.00		\$0.00	13200	\$16,800.00	13200	\$16,800.00		\$0.00		\$0.00		\$0.00
29	Geotextile	sf	\$0.15	\$0.00	\$0.00	\$0.26	\$0.00	\$0.00		\$0.00	13200	\$3,366.00	13200	\$3,366.00		\$0.00		\$0.00		\$0.00
30	Bentonite	ton	\$100.00			\$170.00	\$0.00	\$0.00		\$0.00		\$0.00	96	\$16,320.00		\$0.00		\$0.00		\$0.00
31	mixing	day	\$400.00		\$1,690.42	\$680.00	\$0.00	\$1,848.46		\$0.00		\$0.00	3	\$7,585.29		\$0.00		\$0.00		\$0.00
32	QA Testing					\$0.00	\$0.00	\$0.00		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00
33	soil	allow							allow	\$1,000.00	allow	\$500.00	allow	\$2,500.00	allow	\$1,000.00	allow	\$1,000.00	allow	\$1,000.00
34	geomembrane	allow							allow	\$0.00	allow	\$500.00	allow	\$500.00	allow	\$0.00	allow	\$0.00	allow	\$0.00
35	Vegetation									\$0.00		\$0.00		\$0.00		\$0.00		\$0.00		\$0.00
36	scarification	day	\$200.00		\$940.21	\$340.00	\$0.00	\$924.23	0.5	\$832.12	0.5	\$832.12	0.5	\$832.12	0.5	\$832.12	0.5	\$832.12	0.5	\$832.12
37	seed & mulch	msf	\$50.10	\$12.88	\$13.80	\$85.17	\$17.64	\$15.18	24	\$2,831.76	24	\$2,831.76	24	\$2,831.76	24	\$2,831.76	24	\$2,831.76	24	\$2,831.76
38	Survey Site	day	\$414.75	\$50.00	\$50.00	\$705.08	\$70.00	\$55.00	1	\$830.08	1	\$830.08	1	\$830.08	1	\$830.08	1	\$830.08	1	\$830.08
39	Foreman	day	\$200.00			\$340.00	\$0.00	\$0.00	15.5	\$6,870.00	14.5	\$4,930.00	35.5	\$12,878.00	23.5	\$7,990.00	18.5	\$6,290.00	21.5	\$7,310.00
40	Contingency									\$8,549.86		\$18,721.30		\$29,271.90		\$17,213.68		\$13,983.99		\$13,736.42
41	Total									\$57,299.78		\$100,327.79		\$175,831.42		\$103,282.18		\$83,903.77		\$82,378.49
42	Cost per Square Foot									\$4.77		\$8.36		\$14.64		\$8.61		\$6.99		\$6.86
43	Cost per Square Meter									\$51.48		\$89.99		\$157.54		\$92.64		\$75.26		\$73.89

Costs are based on 1995/1996 values.

Contingency: The contingency cost included in the worksheet above includes unforeseen costs incurred by the contractor while building the test covers. These costs include costs associated with inclement weather, material cost increases, and difficulty with construction beyond that expected. These costs were incorporated into the contractor's original bid as unforeseen costs to be incurred. This is just a verification of actual costs that were actually incurred that did not just add to the profit of the contractor.